



MAGNUM™ BOARD P21 BRACING TEST 2400 X 600



Te Papa Tipu Innovation Park
49 Sala Street
Private Bag 3020
Rotorua
New Zealand
Telephone: +64 7 343 5777
DDI: +64 7 343 5763
Facsimile: +64 7 343 5507
Email: douglas.gaunt@scionresearch.com

Results

To:	Robin Curtis	From:	Doug Gaunt
Organisation:	New Zealand Sustainable Forest Products L.P	Subject:	P21:2010 600mm x 2.4m Wall 9mm Magnum board one side, No Hold Down Brackets
Location:	Christchurch	Date:	19 October 2016
Fax No.:	03 7328 415	No. of	5
Tel No.:	03 7328 414	Pages:	

Please call +64 7 343 5763 if transmission incomplete

Robin

Please find below the results of your three 600mm x 2.4m 9mm Magnum board one side walls as tested on with no hold-downs.

1. BU wind = 46 (77 BU/m) as limited by the ultimate load capacity.
2. BU Earthquake = 41 (68 BU/m) as limited by the ultimate load capacity.

Figures 1, 2 & 3 show the load deflection plots, Figure 4 shows the P21:2010 calculations.

Wall Construction

- 90x45 SG8 studs (600 centres), plates, two rows of nogs
- 9mm Magnum board one side fixed vertically
- 32 mm screws@ 50, 250, then 300mm c/c to studs and 50, 250 to plates plus spot glued at approx 300 centres around edge
- No hold-down brackets
- Tested with M12 hold down bolts and 50x50x3mm washers

RISK AND LIMITATION OF LIABILITY: Scion's liability to the Client arising out of all claims for any loss or damage resulting from this work will not exceed in aggregate an amount equal to two times the Service Fees actually paid by the Client to Scion. Scion will not be liable in any event for loss of profits or any indirect, consequential or special loss or damage suffered or incurred by the Client as a result of any act or omission of Scion under this Agreement.

USE OF NAME: The Client will not use Scion's name in association with the sale and/or marketing of any goods or services

CAUTION

The information contained in this facsimile is confidential and may be legally privileged. If the reader of this message is not the intended recipient, you are hereby notified that any use, dissemination, distribution or reproduction of this message is prohibited. If you have received this message in error, please notify us immediately and return the message to us by mail. Thank you.

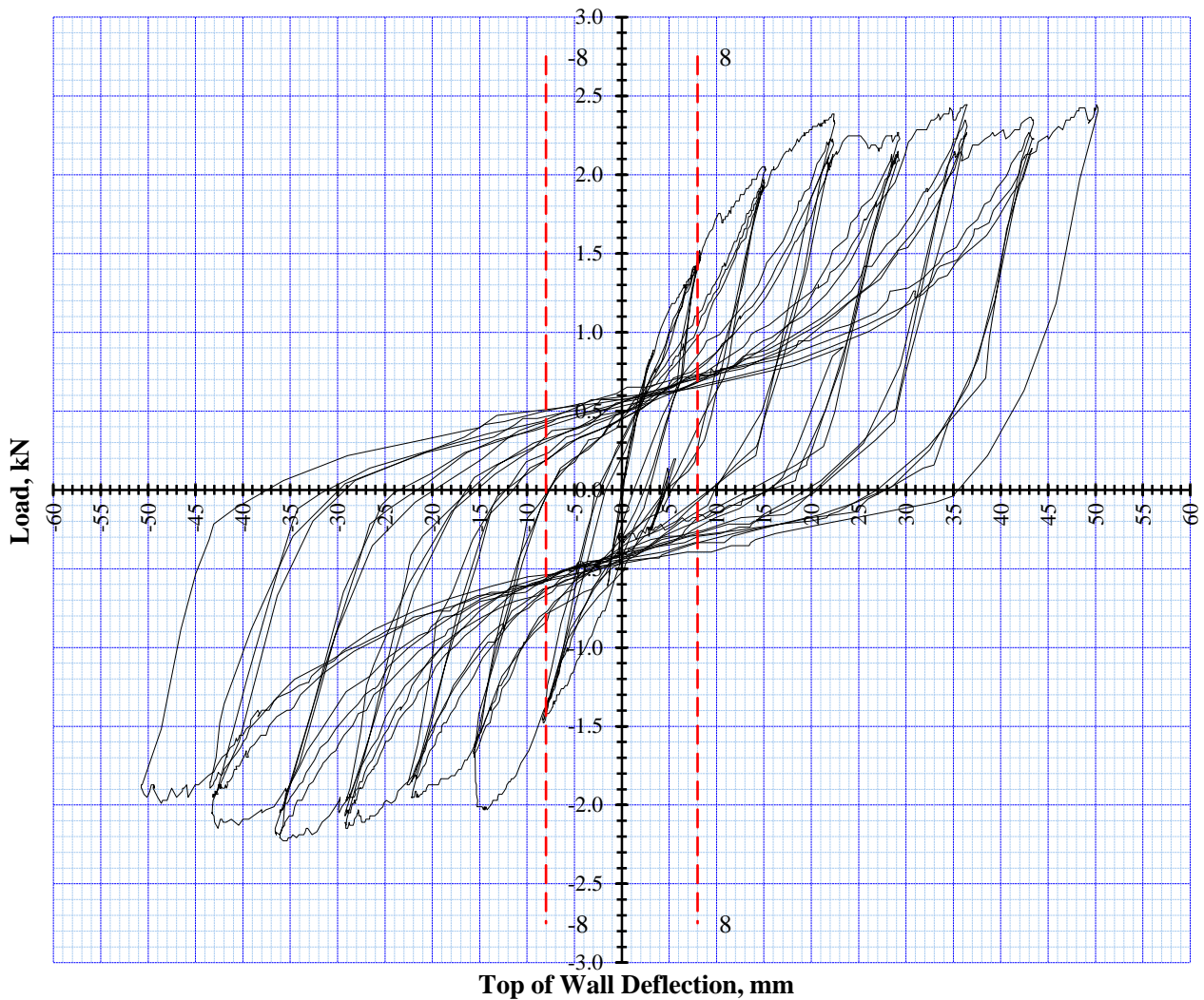


Figure 1: Wall 276256

Wall Condition after test

- The screws remained intact.
- No fractures were seen in the Magnum board for walls 276256 & 276257 however wall 276258 fracture along the bottom plate screw line
- For walls 276256 & 276257 the only apparent damage was a widening of the screw hole through the Magnum board predominantly around those screws on the bottom plate and on the bottom section on the end studs.
- No damage was seen in the timber framing.

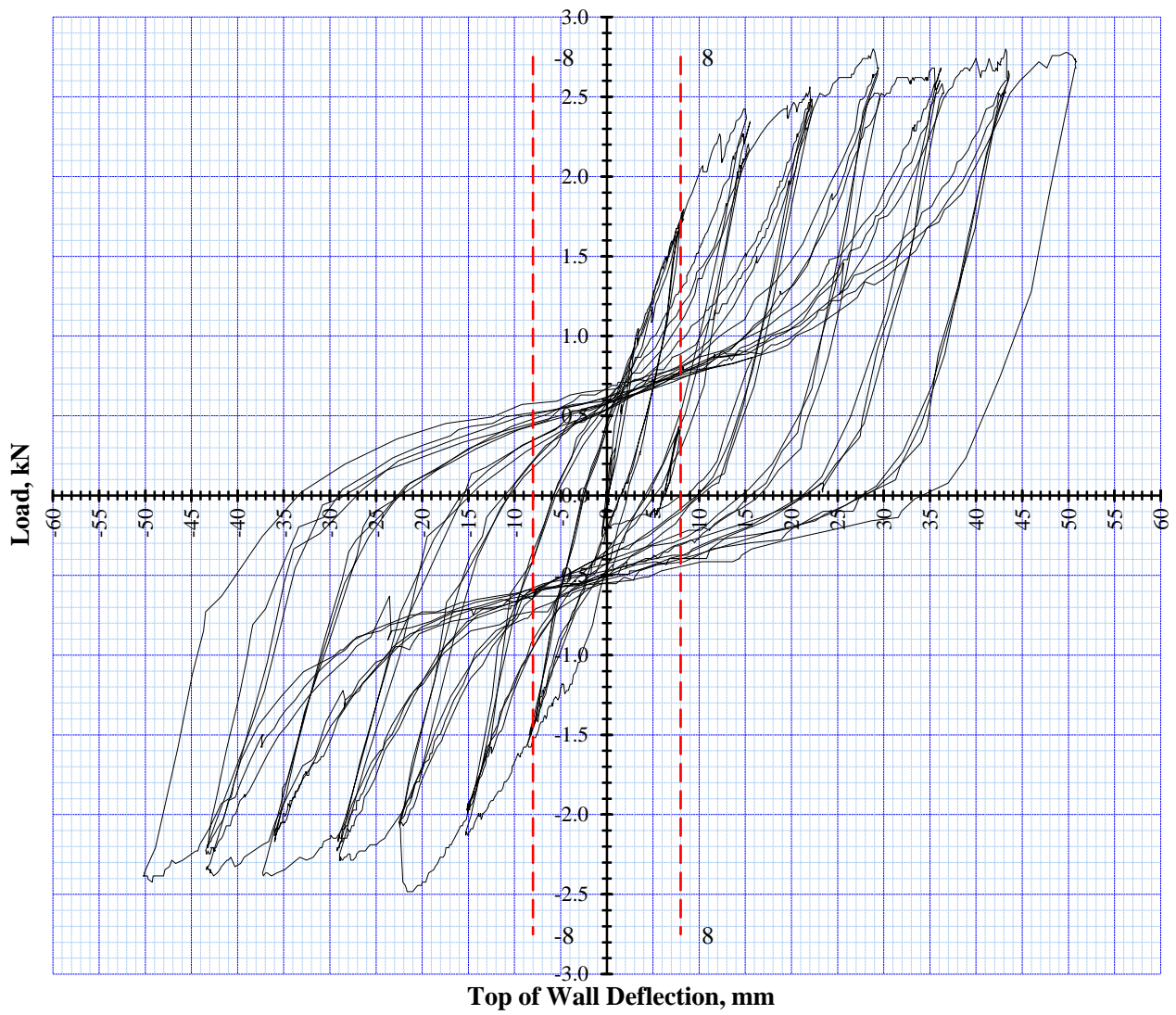


Figure 2: Wall 276257

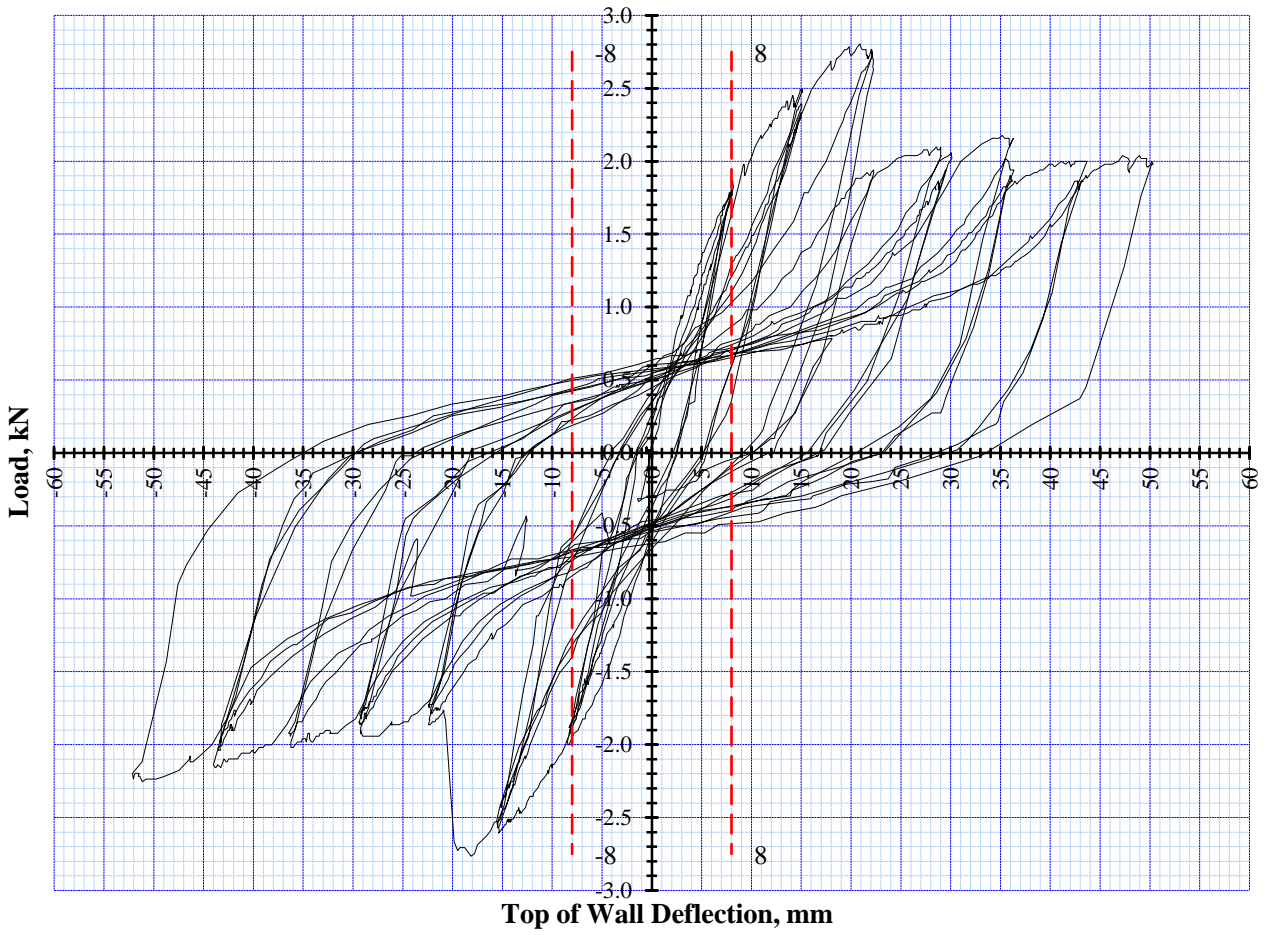


Figure 3: Wall 276258

P21:2010 BRACING RACKING TEST RESULT EVALUATION								
Wall Construction								
600mm, 9mm Magnum board one side using 32mm screws @ 50, 250, then 300mm c/c to studs and 50, 250 to plates plus spot glued at approx 300 centres around edge								
90x45 H1.2 SG8 top and bottom plates, studs 600mm c/cs, two rows of no								
2 x 12mm Hold downs one each end with 50x50x3mm flat washers to bottom plate								
Summary						Earthquake	68 (U)	BU/m
						Wind	77 (U)	BU/m
Date of test:-	19-Oct-16	Ship No.	2921	Tested by		Doug Gaunt		
Date of calc's:-	19-Oct-16	Job No.	TE16-028	Analysed by		Doug Gaunt		
Calculated to BRANZ P21:2010, AS/NZS1170.2&5, NZS3604:2010 Scion, Private Bag 3020 Rotorua.								
Serviceability Cycles			Ultimate Cycles					
Lab Number	Direction	Cycle to H/300 or DLQ or DLW		Cycle to Displacement		Wall dimensions		
		8.0	X mm	y=(mm)		L(mm)	H(mm)	
		Loads	Residual	Maximum				
		(P ₈)	Defln, C	Load	def @ P			
		kN	mm	P(kN)	y (mm)	P/2 (kN)	d mm	4th,R
								kN
276256	+	1.42	2.00	2.43	36.0	1.22	6.4	2.15
	-	1.43	2.00	2.23	36.0			1.95
276257	+	1.75	1.80	2.67	36.0	1.34	5.4	2.40
	-	1.55	2.60	2.37	36.0			2.06
276258	+	1.80	2.30	2.16	36.0	1.08	3.8	1.83
	-	1.93	1.50	2.02	36.0			1.78
		(P ₈)	(C)	(P)	(y)	P/2 (kN)	(d)	(R _y)
Averages		1.65	2.03	2.31	36.00	1.21	5.20	2.03
Coefficient of Variation %		11.68	17.19	9.01	0.00	8.61	20.59	10.28
y = average failure deflection or peak deflection of the three tests.								
d= average first cycle displacement at half peak, (the very first cycle wall reaches the load)								
R = Residual load, P = Peak Load, S = Serviceability load								
Displacement Recovery Factor (K1), (0.8 <= K1 <= 1.0)					Systems factor K2 = 1.2			
Average Structural Displacement Ductility factor					u = y/d 6.92			
Ductility Modification factor					K4 = 1.00			
DLW = Selected deflection limit for wind forces				DLQ = Selected deflection limit for earthquake forces				
P21:2010 BR Calc's		K1	EQ ultimate	EQ service	Wind Ultimate	Wind Service		
Lab Number		(= 1.4 - C/X)	BU's	BU's	BU's	BU's		
276256	(BU)	1.00	41.0	62.2	46.6	48.2		
	(BU/m)		68	104	78	80		
276257	(BU)	1.00	44.6	72.0	50.4	55.8		
	(BU/m)		74	120	84	93		
276258	(BU)	1.00	36.1	81.4	41.8	63.0		
	(BU/m)		60	136	70	105		
<20% Result Check		276256	2% Ok result	-23% Ok result	1% Ok result	-23% Ok result		
		276257	14% Ok result	0% Ok result	12% Ok result	0% Ok result		
		276258	-19% Ok result	74.6	-16% Ok result	57.8		
Note: Where the value of BR Wind or BR EQ for any specimen is more than 20% greater than either of the other two specimens, assign it a value of 1.2 times the lower value before averaging.								
Average Earthquake BR			Ultimate			Serviceability		
EQ (BU's)	20 x K4 x R _y =		41	(P8 x K1) x (K2/0.55) =		70		
			68 BU/m	Limited by		Ultimate limit state		
Average Wind BR			Ultimate			Serviceability		
Wind (BU's)	20 * P =		46	(P8 x K1) x (K2/0.71) =		54		
			77 BU/m	Limited by		Ultimate limit state		

Figure 4: P21:2010 calculations for a 600mm x 2.4m 9mm Magnum board one side wall

Please feel free to contact me to discuss this information.

Doug Gaunt

